

**SOSIALISASI LEBIH DALAM ISI SNI 1726
GeoTalk XIX 22
Zoom Webinar, 11-12 Juli 2022**

PERKEMBANGAN PETA GEMPA DAN PENERAPANNYA DALAM PERENCANAAN INFRASTRUKTUR DAN GEDUNG TAHAN GEMPA INDONESIA

Prof. Dr. Ir. Masyhur Irsyam* dkk

*Ketua - Tim Pemutakhiran Peta Gempa Indonesia 2010 dan 2017
*Koordinator - Tim Struktur Bawah Revisi SNI 1726:2019
*Ketua Umum - Himpunan Ahli Teknik Tanah Indonesia (2011-2019)
*Koordinator - Akademi Ilmu Pengetahuan Indonesia AIPI

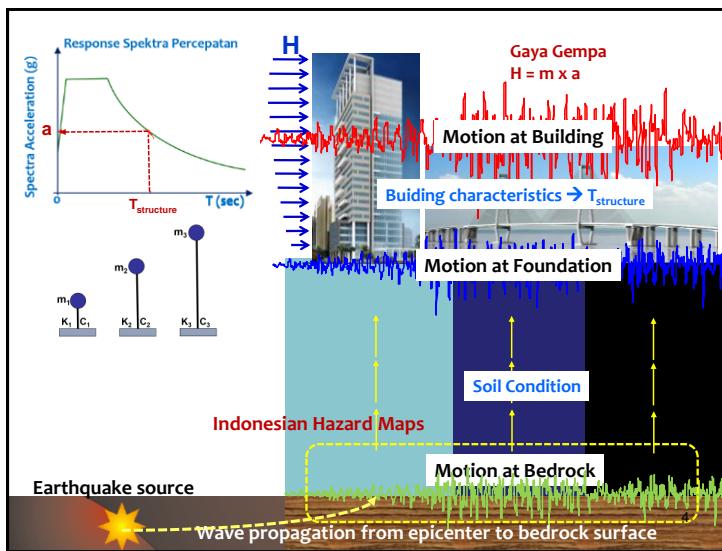
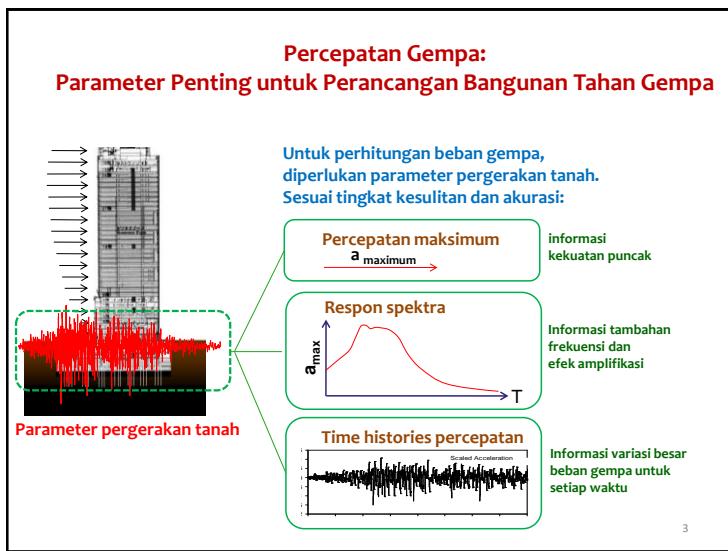
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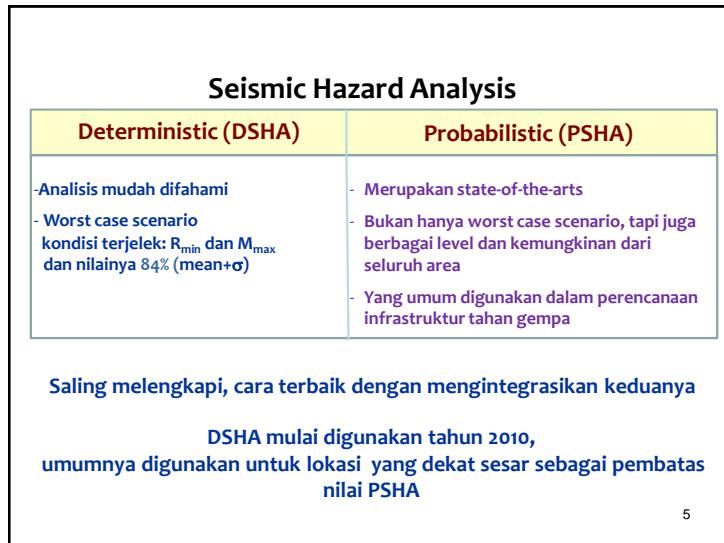
**SOSIALISASI LEBIH DALAM ISI SNI 1726
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Cakupan Materi Bahasan:

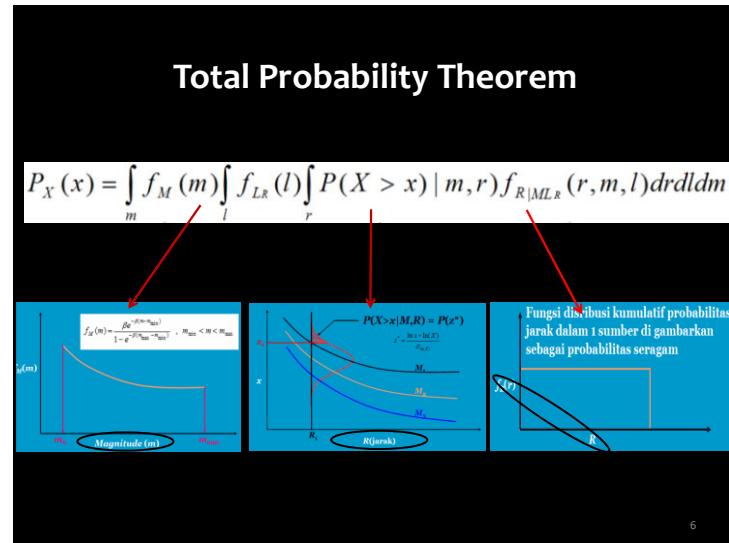
- Penyusunan Peta Sumber dan Bahaya Gempa Nasional Indonesia 2017
- Aplikasi Peta Gempa dalam Perencanaan Infrastruktur Tahan Gempa Indonesia
- Aplikasi Peta Gempa untuk Perencanaan Gedung dalam SNI 1726-2019

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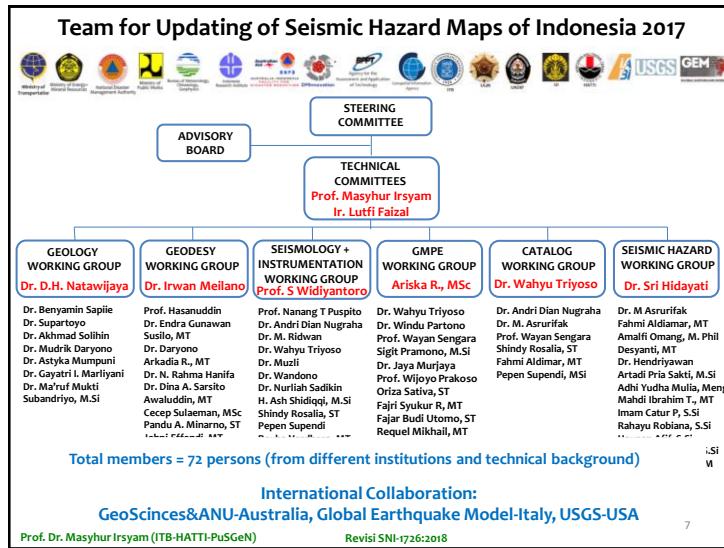




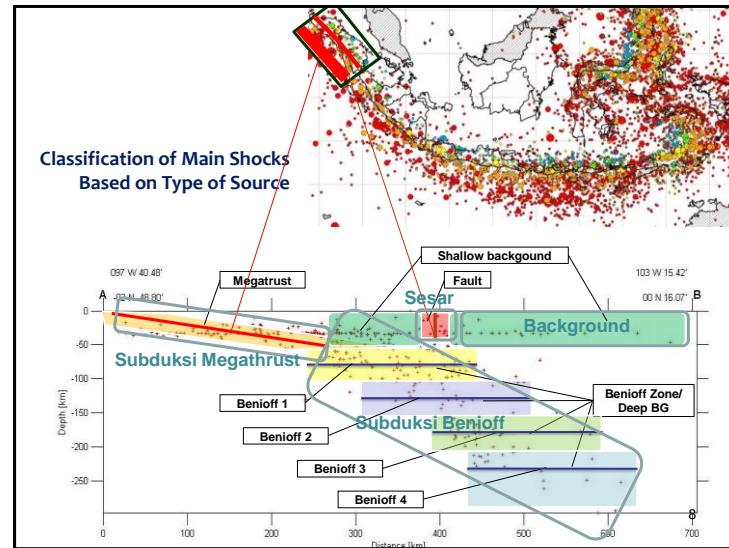
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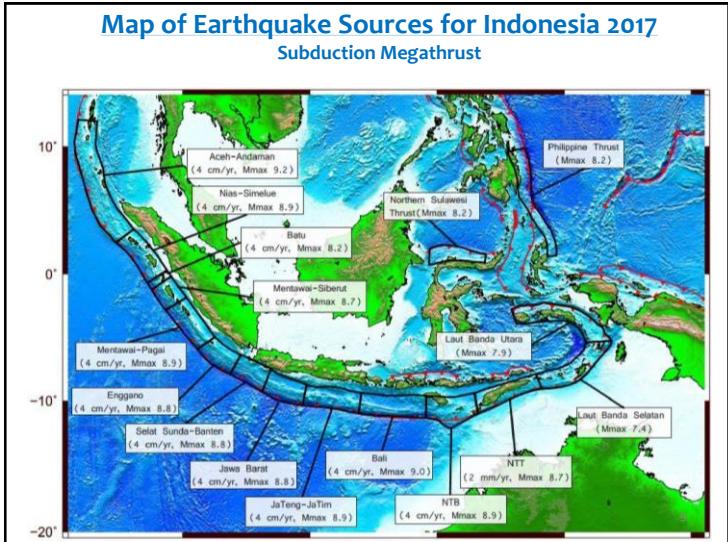
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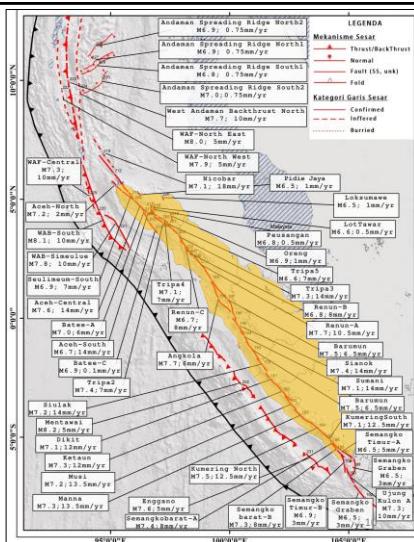
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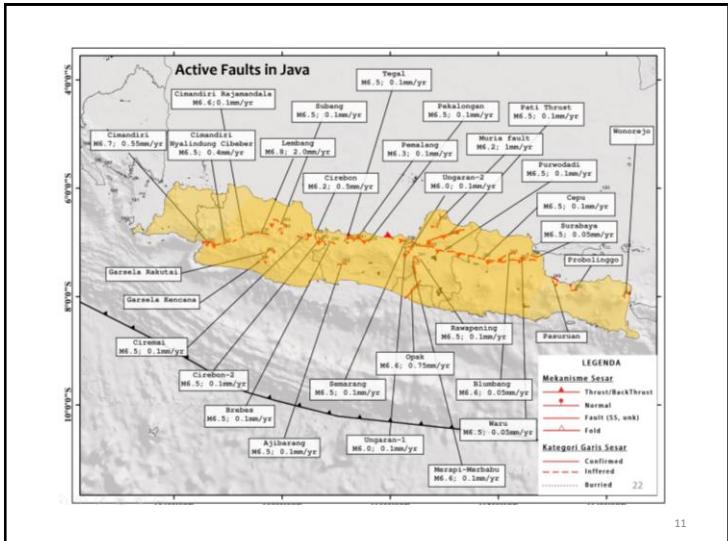
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Penyempurnaan oleh Pakja Geologi dengan merevisi lokasi dari jalur sesar aktif yang sudah dipetakan sebelumnya dan menambahkan banyak jalur-jalur sesar aktif yang belum diperhitungkan dalam peta 2010.

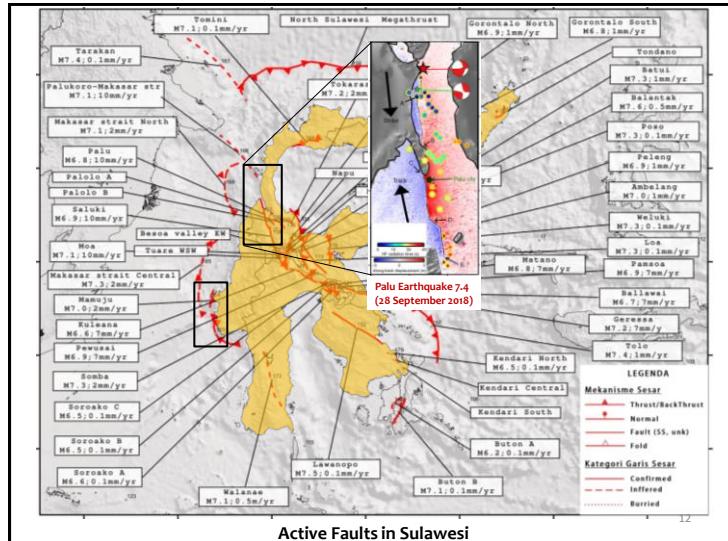
Hazard Maps 2010: 81 active fault lines
Hazard Maps 2017: 295 active fault lines



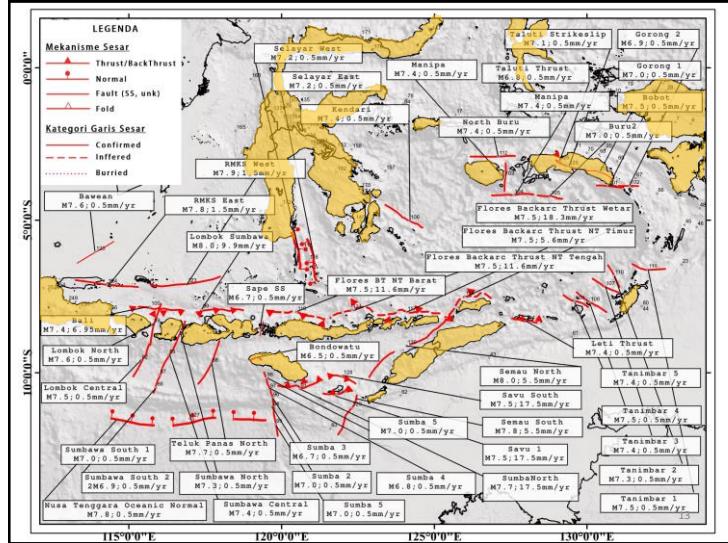
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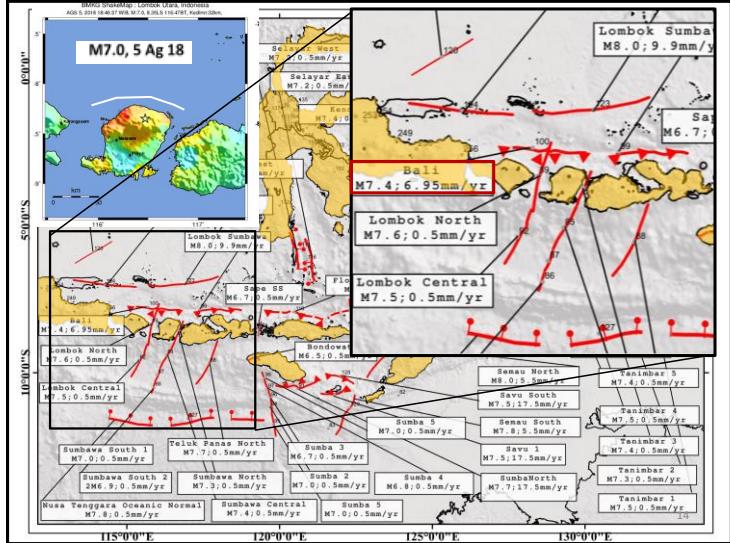
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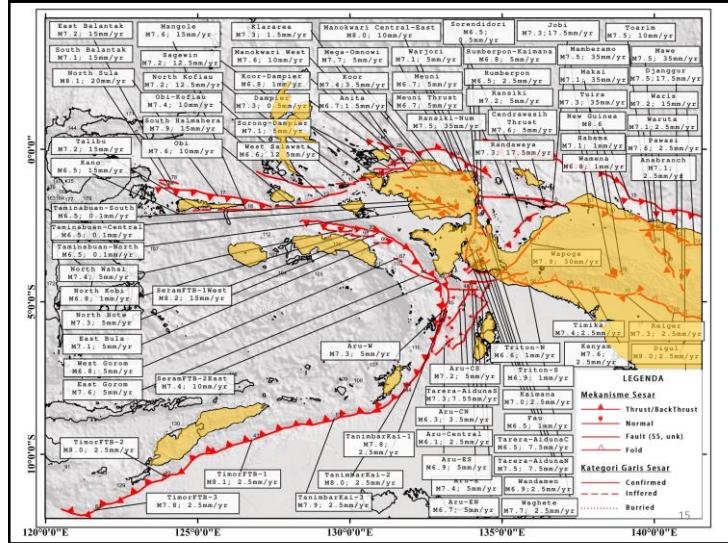
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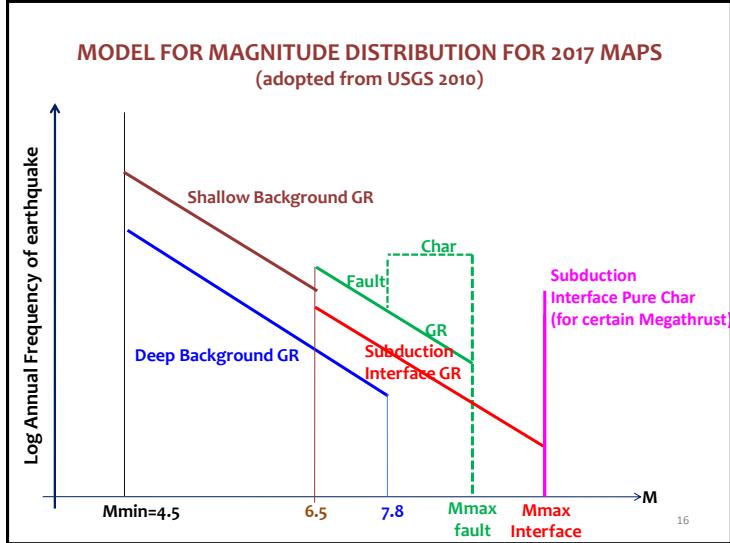
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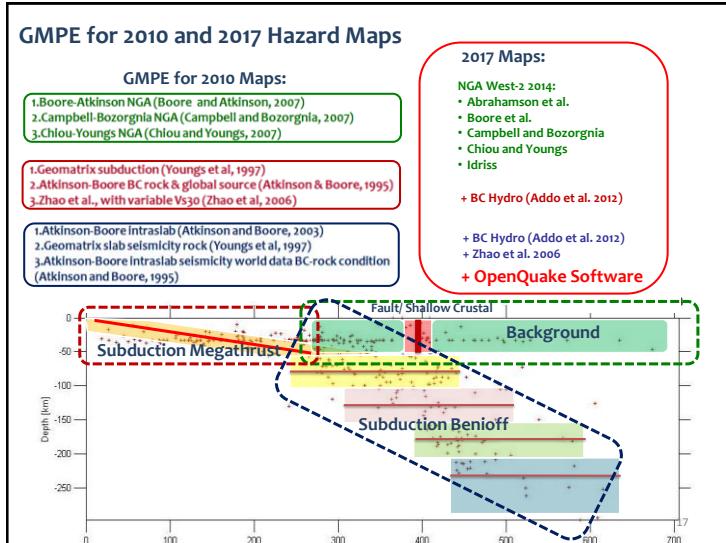
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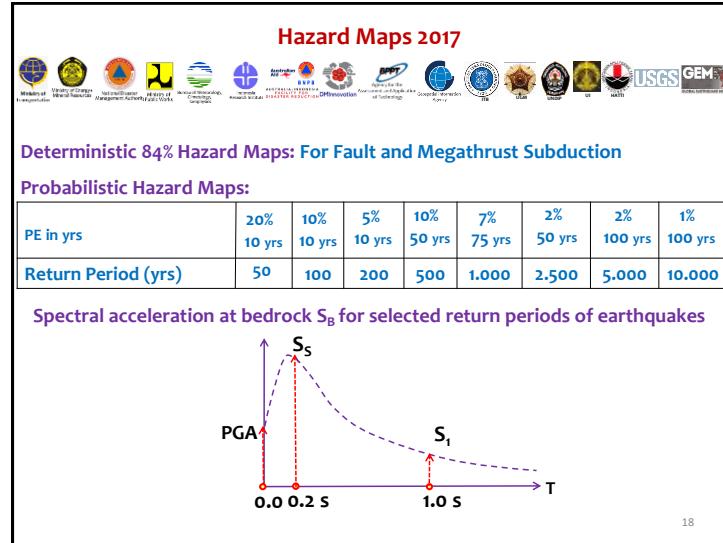
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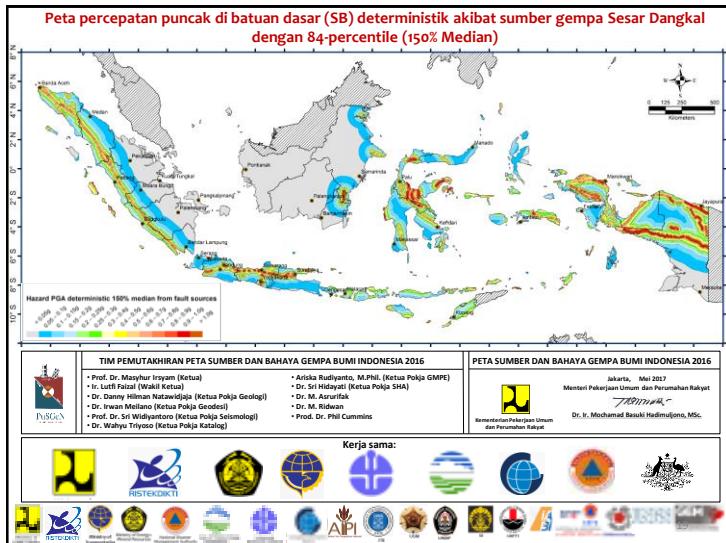
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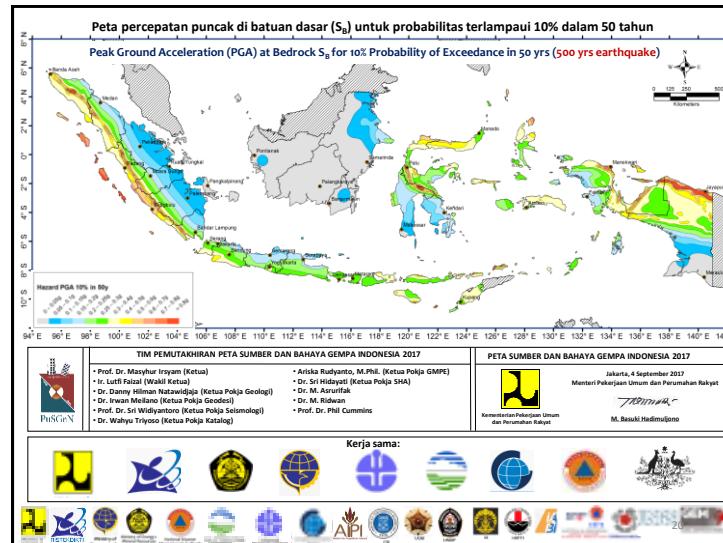
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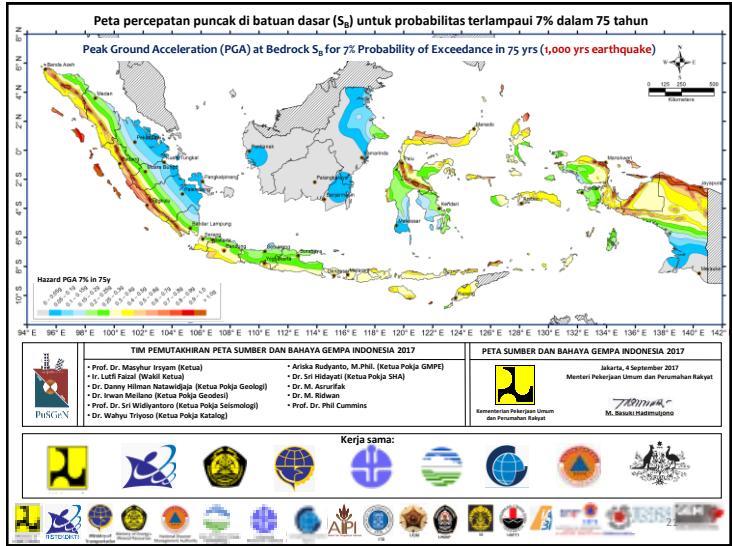
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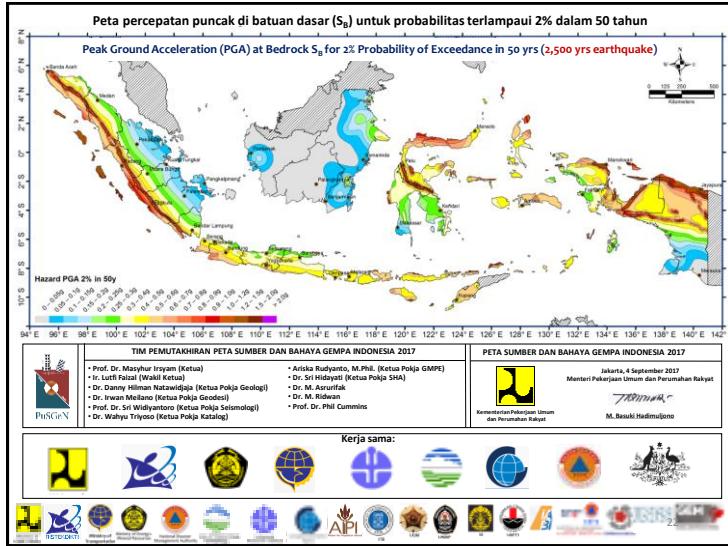
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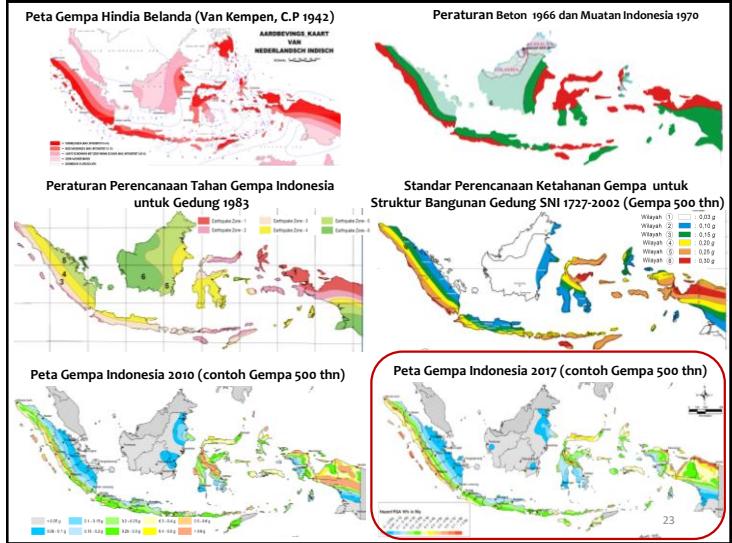
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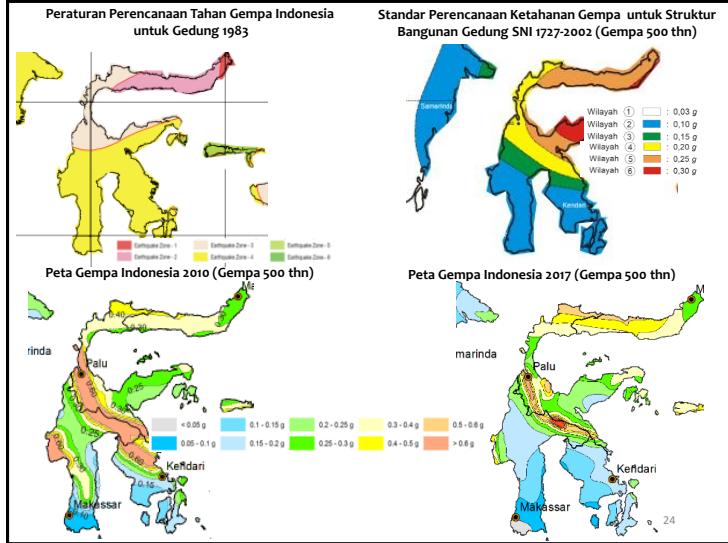
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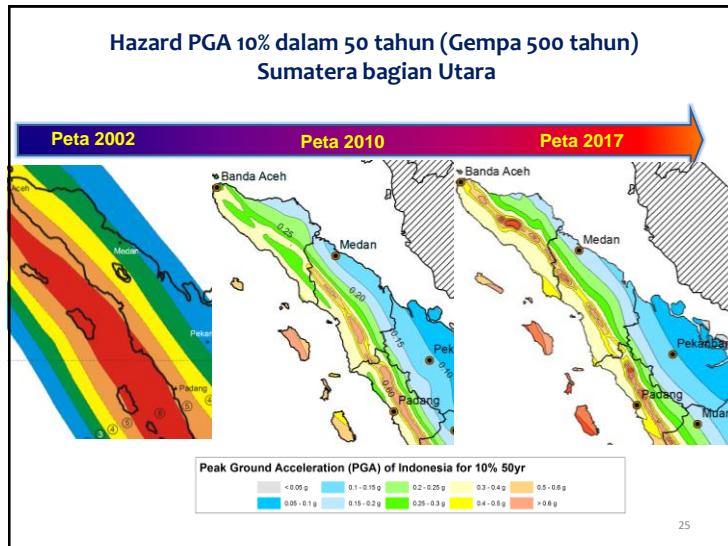
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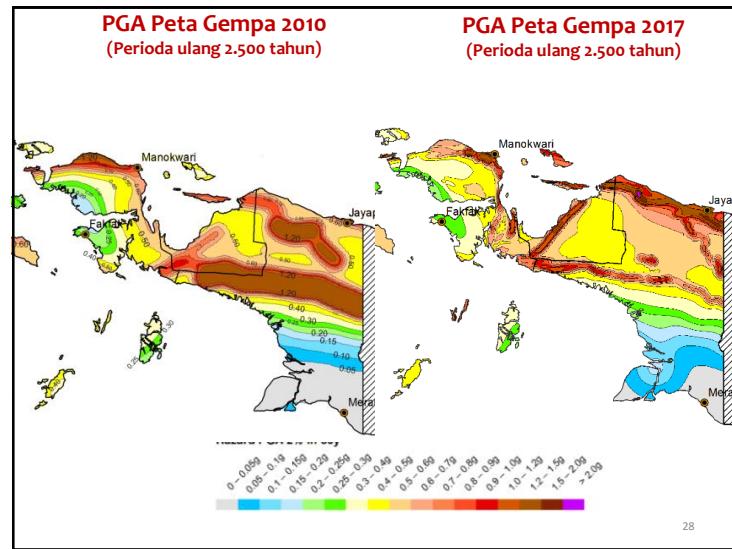
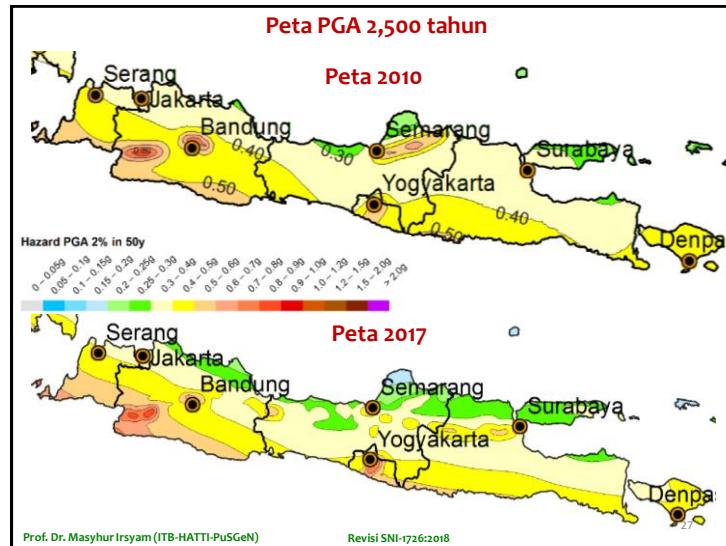
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SNI GEOTEKNIK SNI 8460:2017
Tabel 52 – Kriteria Perancangan Gempa Berdasarkan Peruntukan Infrastruktur

Peruntukan	Periode ulang	Umur rencana	Probabilitas terlampaui	Kriteria keamanan	Referensi
Bangunan gedung dan non-gedung	2.500 Tahun	50 Tahun	2% Kemungkinan Terlampaui/ 1% Risiko Runtuh	-	SNI 1726:2012 SNI 1726-2019
Jembatan konvensional	1.000	75	7%	-	SNI 2833:2016 AAHTO (2012)
Dinding penahan abutmen jembatan	1.000	75	7%	- FK>1,5 (terhadap geser saat mengalami beban statik) - FK>2 (terhadap guling saat mengalami beban statik) - FK>1,1 (terhadap beban pseudostatik)	WSDOT, FHWA-NJ-2005-002
Timbunan oprit				FK>1,1	
Bendungan	10.000 Safety Evaluation Earthquake (SEE)	100	1%	Tidak terjadi aliran air yang tidak terkendali - Deformasi tidak melebihi 0,5 dari tinggi jagung - Deformasi pada filter tidak boleh melebihi 0,5 total filter - Spillway tetap berfungsi setelah terjadi gempa rencana	ICOLD No 148, 2016,
	145 Operating Basis Earthquake (OBE)	100	50%	- Kerusakan minor setelah terjadi gempa rencana	
Bangunan pelengkap bendungan	2500	50	2%	-	
Terowongan	1.000 tahun	100	10%	-	31

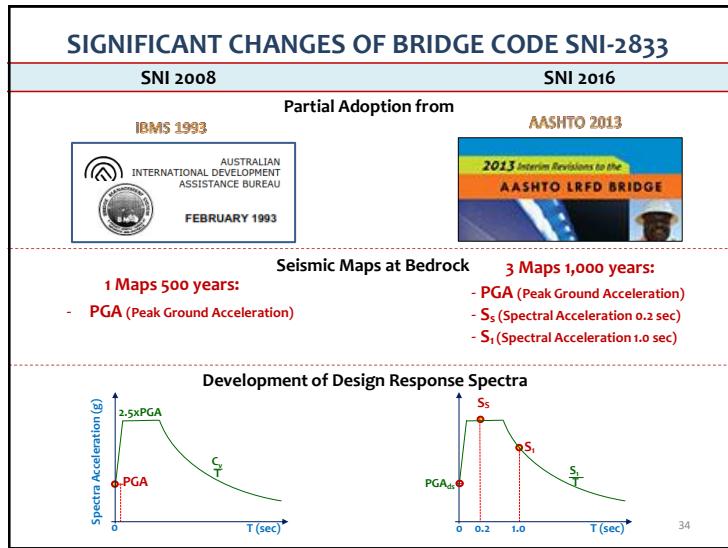
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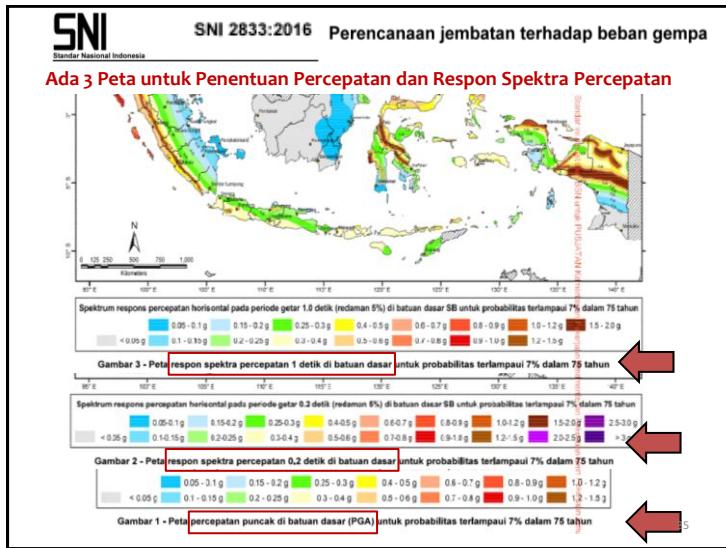
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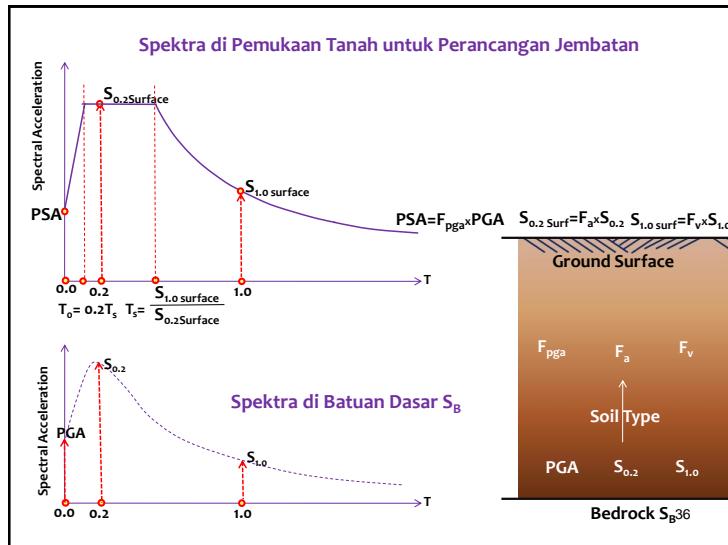
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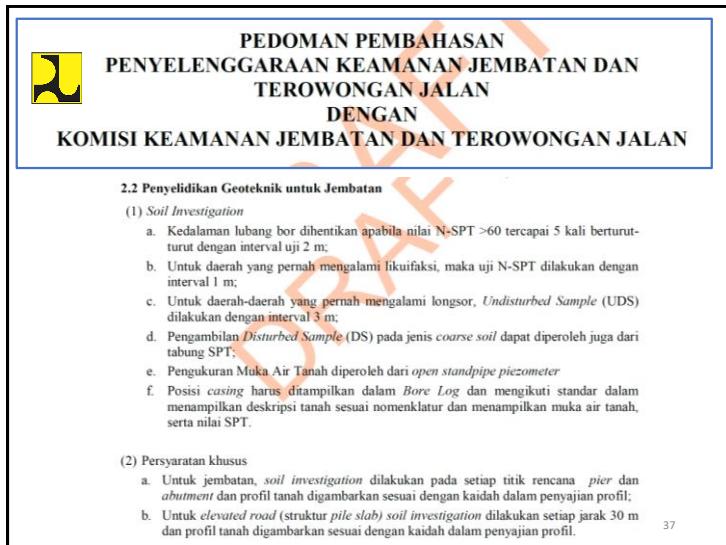
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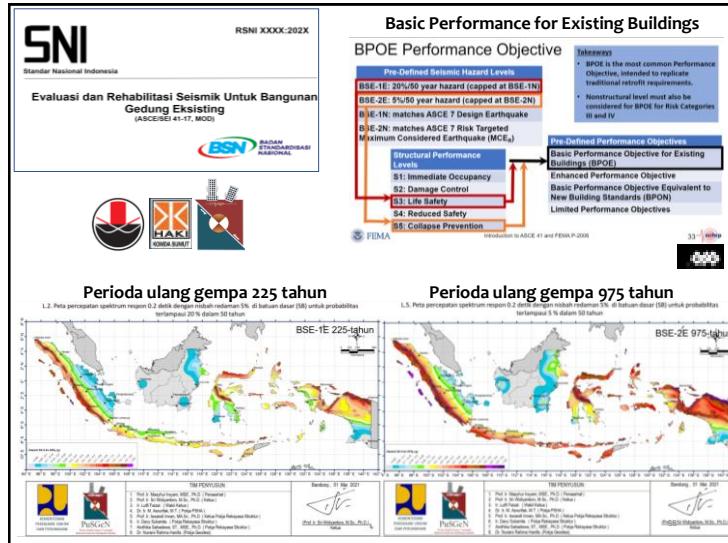
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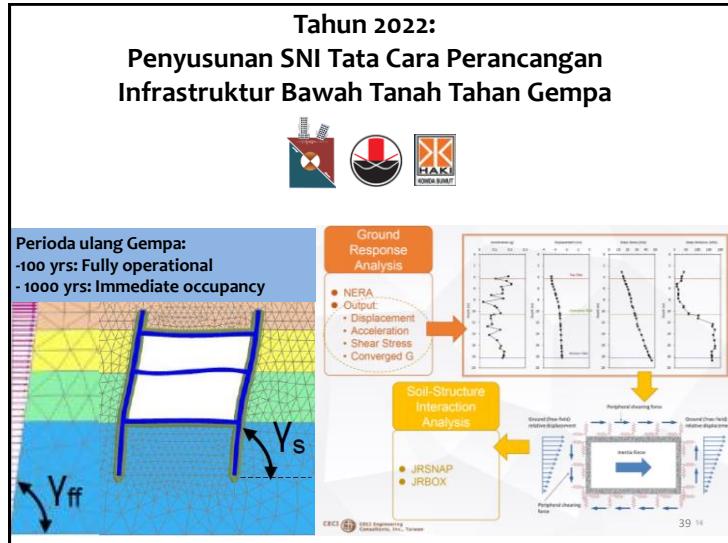
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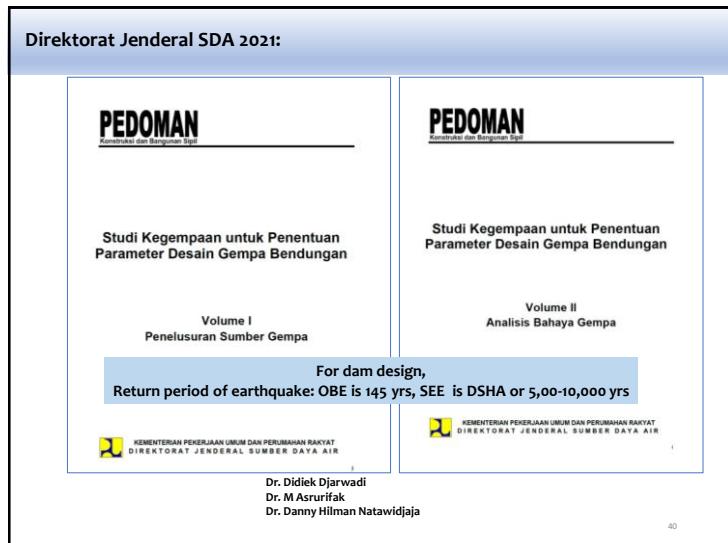
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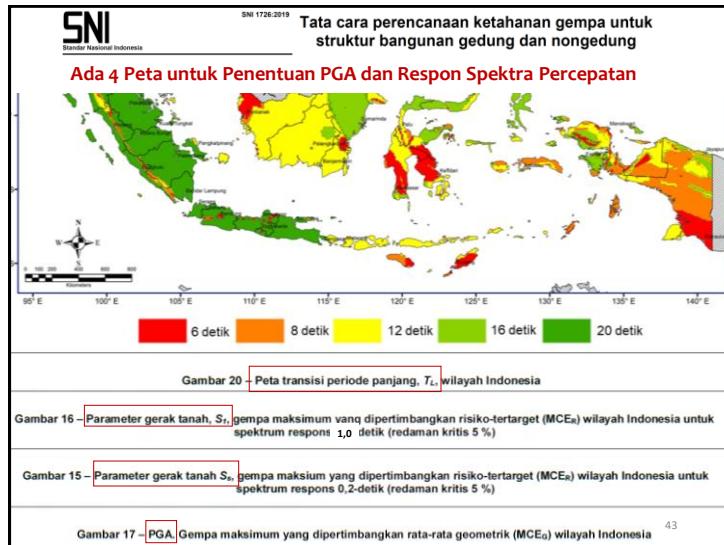
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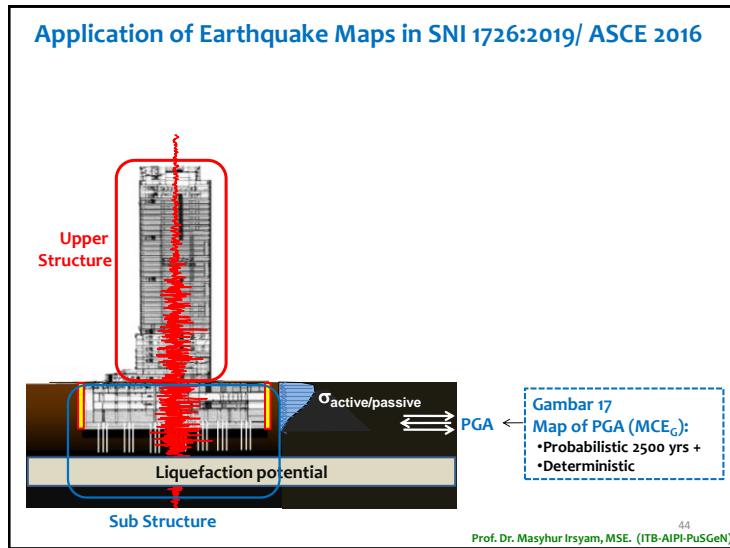
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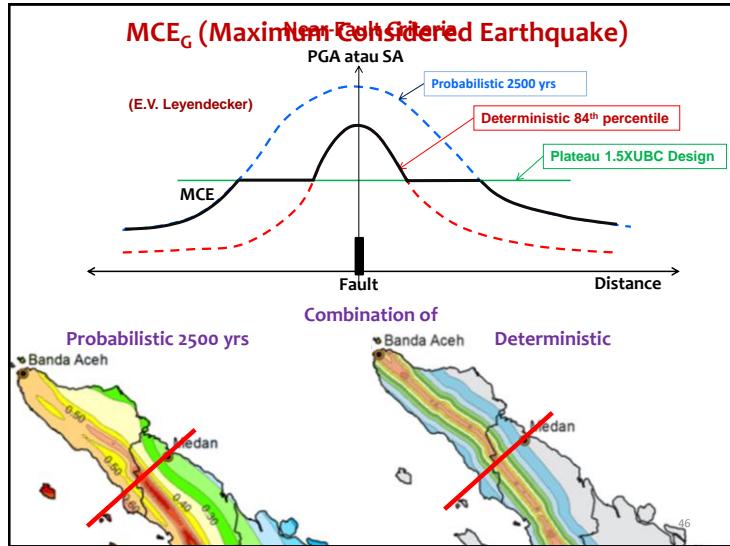
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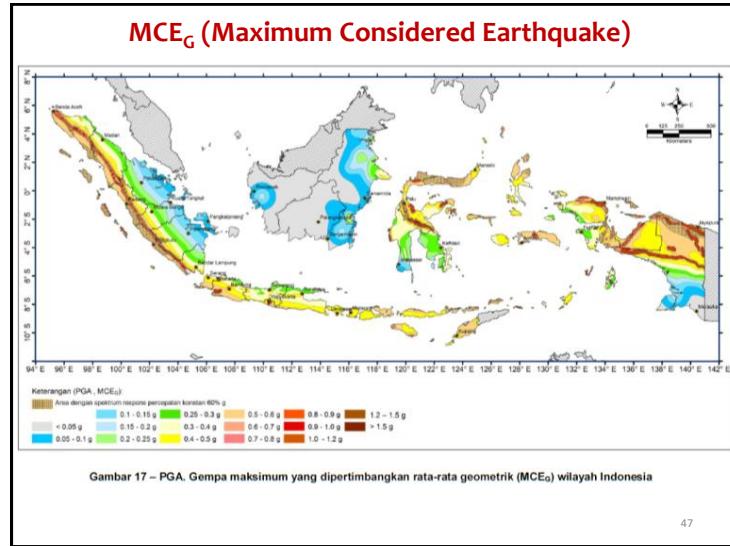
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SNI 2002	SNI 2012	SNI 2019
Gambar 2.1	Gambar 11	Gambar 17
Criteria for PGA Maps		
<ul style="list-style-type: none"> • Probabilistic: 10% PoE in 50 yrs (500 yrs) • Probabilistic: 2% PoE in 50 yrs (2,500 yrs) • Probabilistic: 2% PoE in 50 yrs (2,500 yrs) <ul style="list-style-type: none"> • Deterministic 84% PGA • Deterministic 84% PGA • Deterministic 84% PGA 		

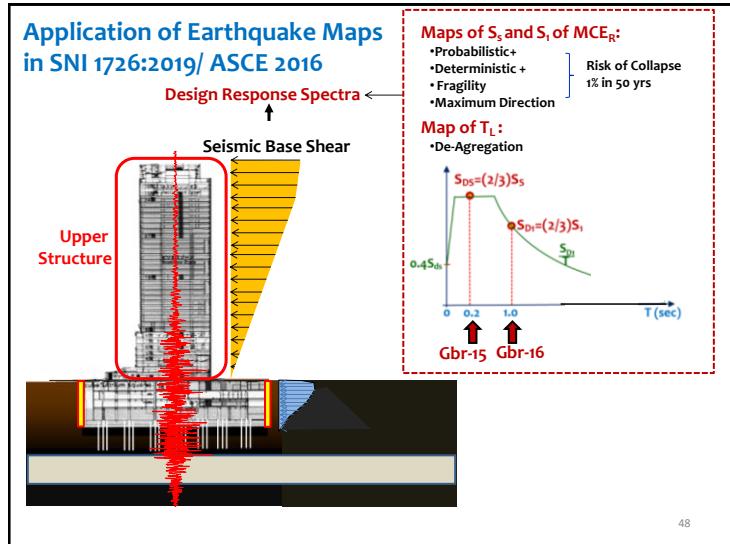
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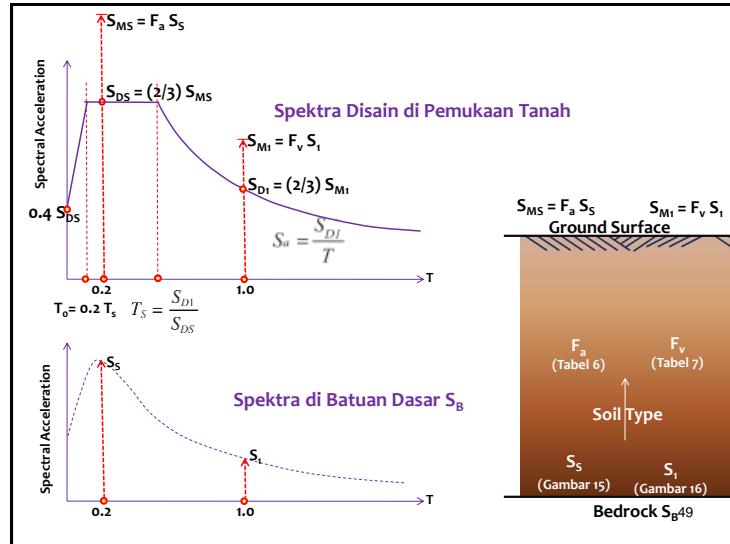
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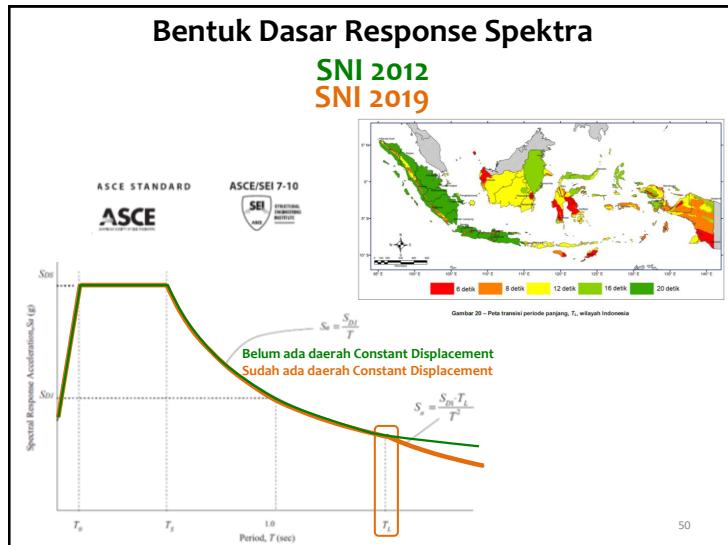
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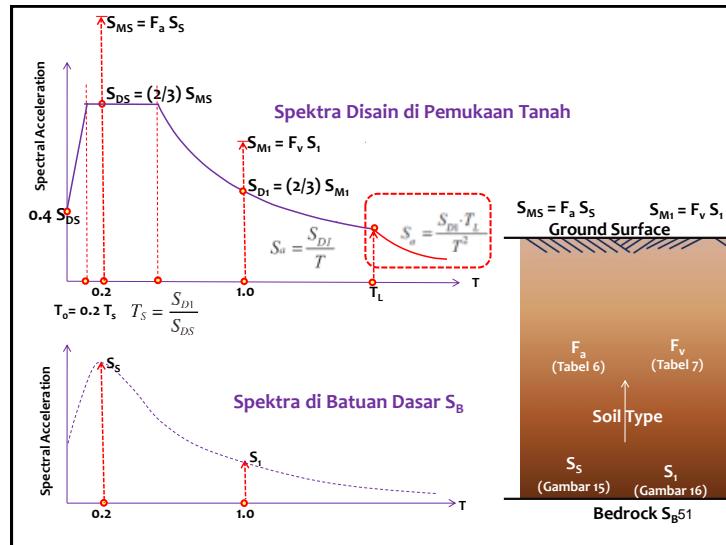
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Short-Period Site Coefficient F_a (Tabel 6, Pasal 6.2)

SNI 2019 follows PEER 2013

Site Class	$S_s < 0.25$			$S_s = 0.5$			$S_s = 0.75$		
	SNI 2012	ASCE 7-16	PEER	SNI 2012	ASCE 7-16	PEER	SNI 2012	ASCE 7-16	PEER
A	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8
B	1.0	0.9	0.9	1.0	0.9	0.9	1.0	0.9	0.9
C	1.2	1.3	1.3	1.2	1.3	1.3	1.1	1.2	1.2
D	1.6	1.6	1.6	1.4	1.4	1.4	1.2	1.2	1.2
E	2.5	2.4	2.4	1.7	1.7	1.7	1.2	1.3	1.3

Site Class	$S_s = 1.0$			$S_s = 1.25$			$S_s > 1.5$		
	SNI 2012	ASCE 7-16	PEER	SNI 2012	ASCE 7-16	PEER	SNI 2012	ASCE 7-16	PEER
A	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8
B	1.0	0.9	0.9	0.9	0.9	0.9	1.0	0.9	0.9
C	1.0	1.2	1.2	1.2	1.2	1.2	1.0	1.2	1.2
D	1.1	1.1	1.1	1.0	1.1	1.1	1.0	1.0	1.0
E	0.9	SS	1.1	0.9	SS	0.9	0.9	SS	0.8

SS: Site Specific Response Analysis

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Long-Period Site Coefficient F_v (Tabel 7, Pasal 6.2)

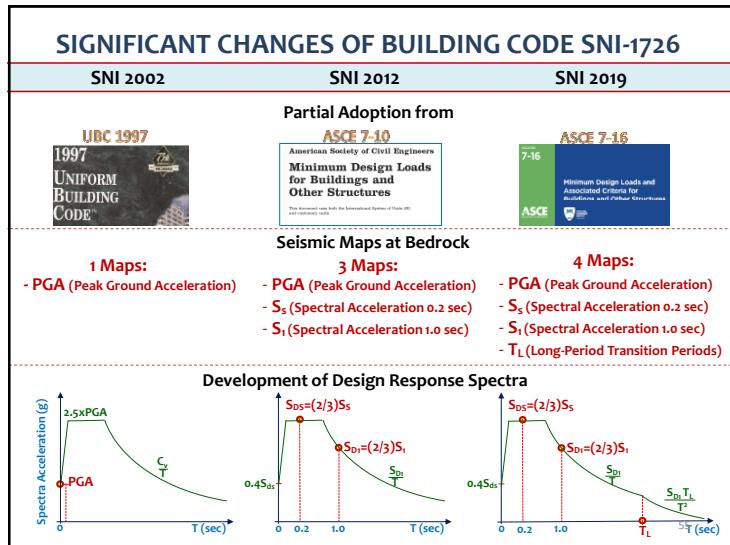
SNI 2018 follows PEER 2013

Site Class	$S_i < 0.1$			$S_i = 0.2$			$S_i = 0.3$		
	SNI 2012	ASCE 7-16	PEER	SNI 2012	ASCE 7-16	PEER	SNI 2012	ASCE 7-16	PEER
A	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8
B	1.0	0.8	0.8	1.0	0.8	0.8	1.0	0.8	0.8
C	1.7	1.5	1.5	1.6	1.5	1.5	1.5	1.5	1.5
D	2.4	2.4	2.4	2.0	2.2	2.2	1.8	2.0	2.0
E	3.5	4.2	4.2	3.2	SS	3.2	2.8	SS	2.8

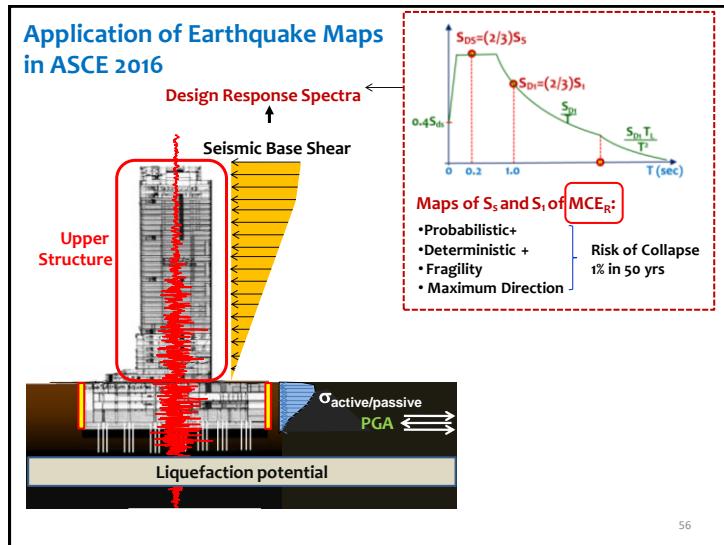
Site Class	$S_i = 0.4$			$S_i = 0.5$			$S_i > 0.6$		
	SNI 2012	ASCE 7-16	PEER	SNI 2012	ASCE 7-16	PEER	SNI 2012	ASCE 7-16	PEER
A	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8	0.8
B	1.0	0.8	0.8	1.0	0.8	0.8	1.0	0.8	0.8
C	1.4	1.5	1.5	1.3	1.5	1.5	1.3	1.4	1.4
D	1.6	1.9	1.9	1.5	1.8	1.8	1.5	1.7	1.7
E	2.4	SS	2.4	2.4	SS	2.2	2.4	SS	2.0

SS: Site Specific Response Analysis

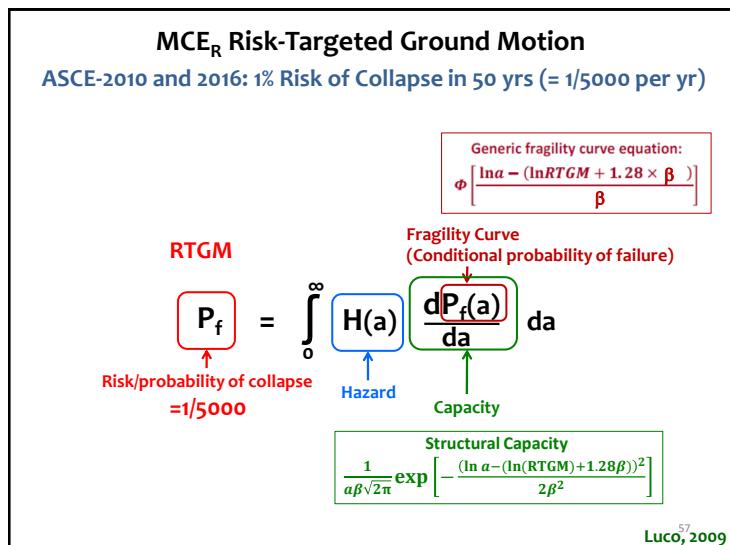
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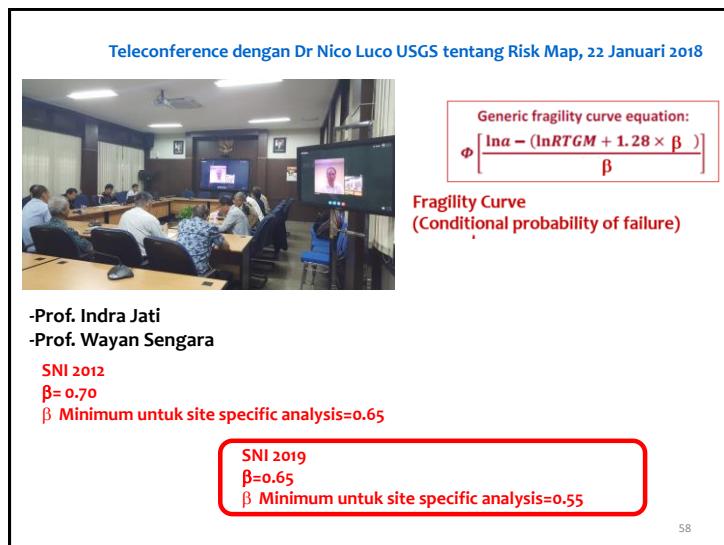
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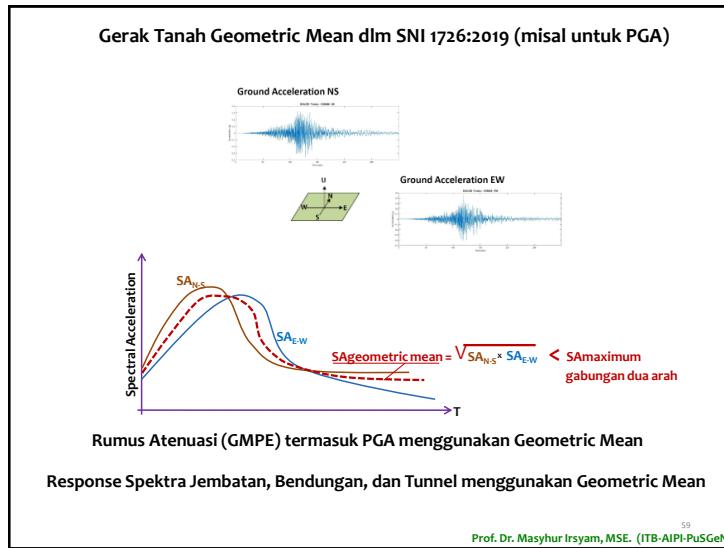
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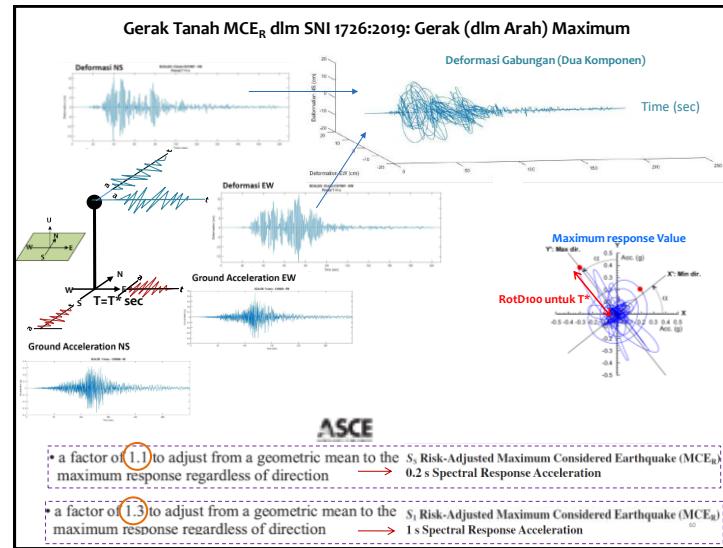
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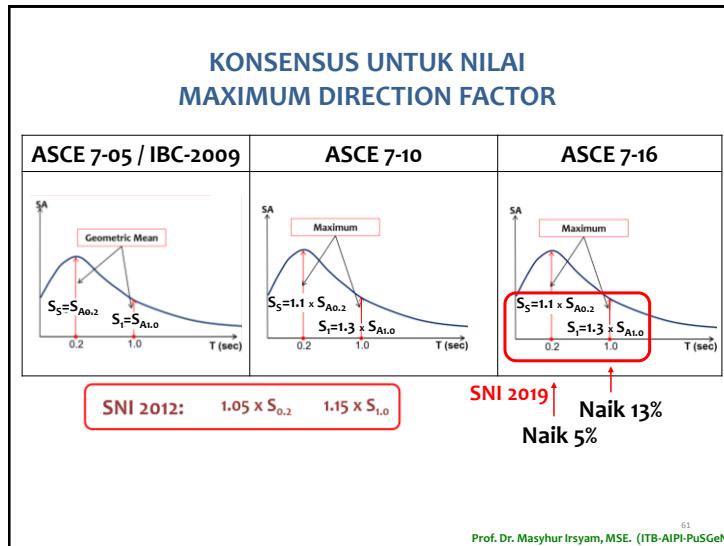
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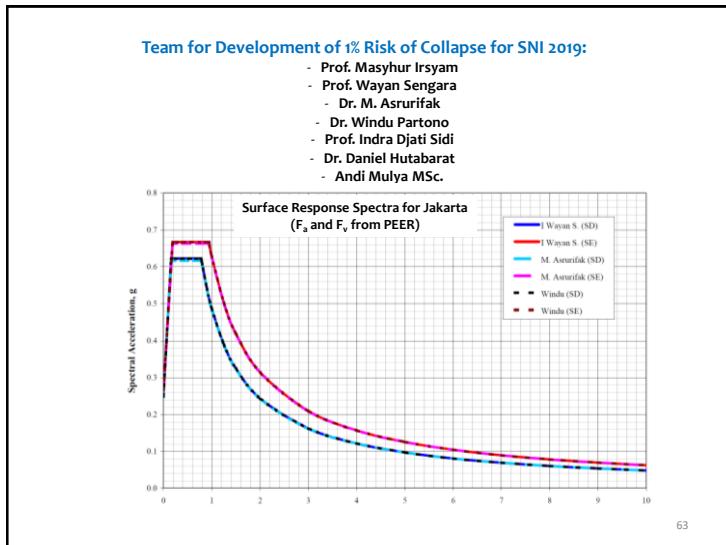
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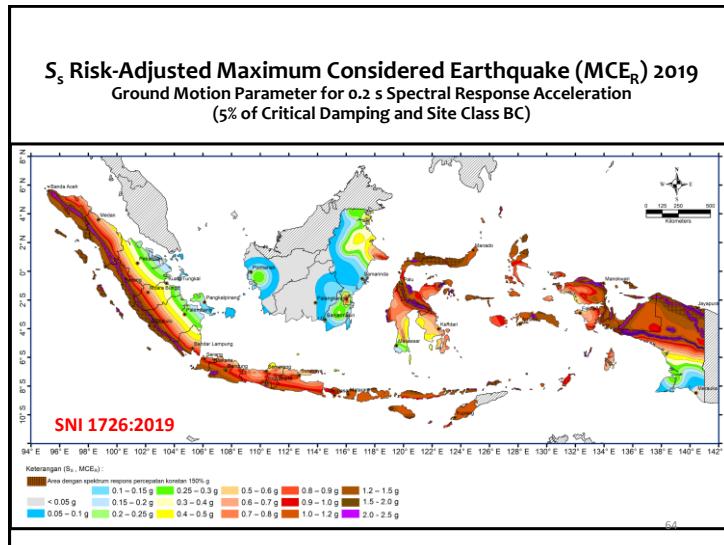
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SNI 2002	SNI 2012	SNI 2019
Criteria for Seismic Maps for Development of Design Response Spectra		
10% Probability of Exceedance in 50 yrs	1% Risk of Collapse in 50 yrs	1% Risk of Collapse in 50 yrs
Developed based on		
- Probabilistic SHA	- Probabilistic SHA - Deterministic SHA - Fragility function ASCE	- Probabilistic SHA - Deterministic SHA - Fragility function ASCE - De-Aggregation (for T_L)
No fragility function	ASCE generic fragility curve equation $\beta = 0.70$	$\beta = 0.65$
Directivity Factors for Maximum Directions		
- No directivity factor	- 1.05 for S_S - 1.15 for S_I (average of UBC 2009 + ASCE 2010)	- 1.10 for S_S - 1.30 for S_I (follows ASCE 2010, 2016)
Increase in S_S = 5% Increase in S_I = 13%		

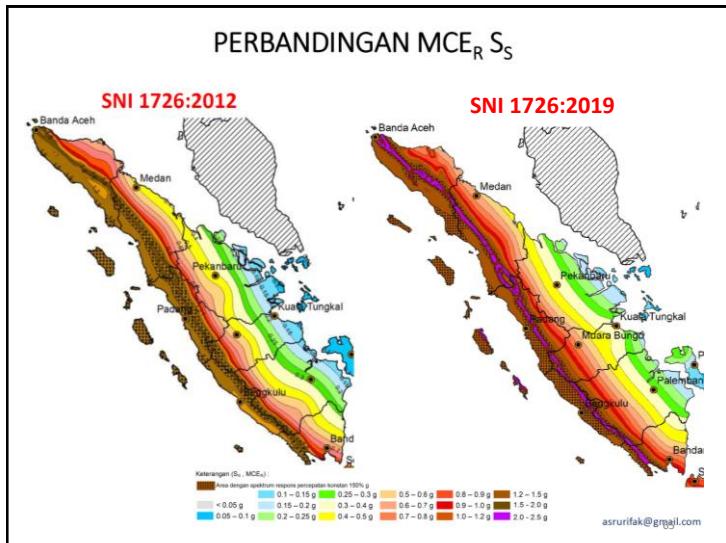
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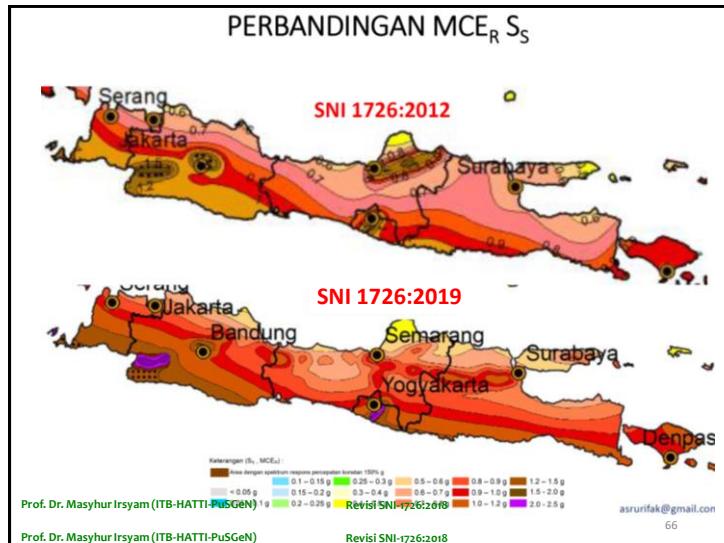
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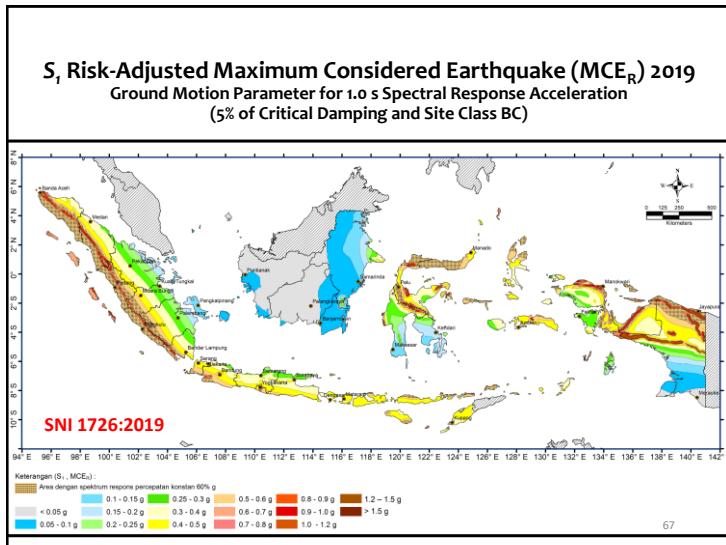


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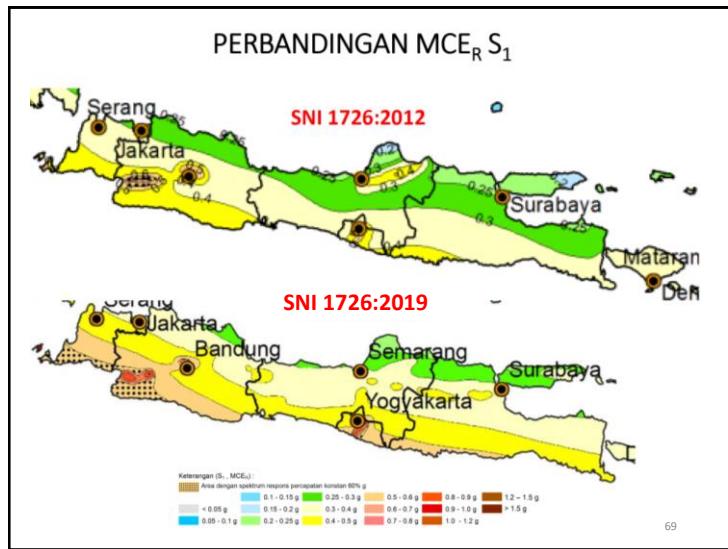
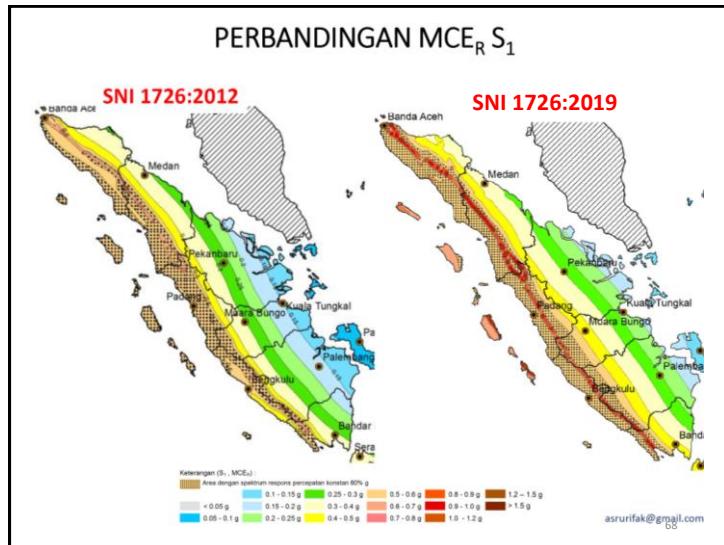


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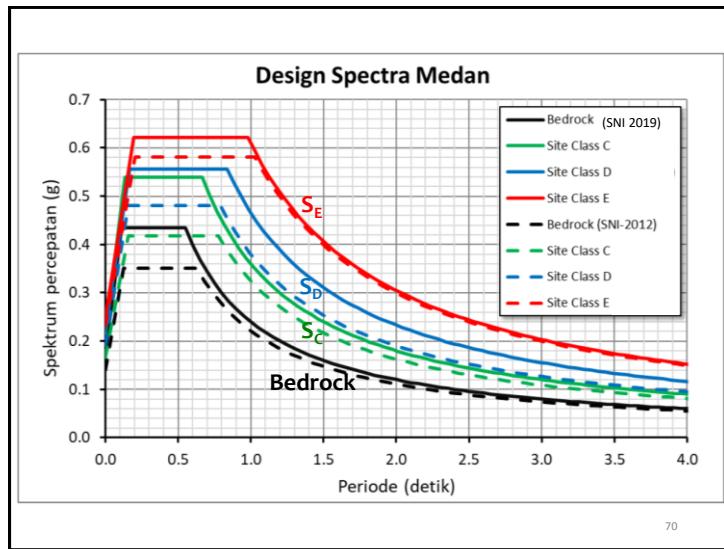




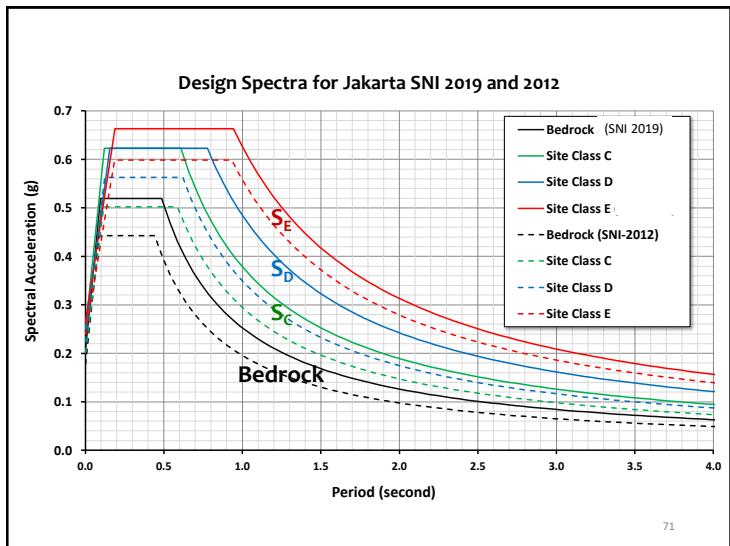
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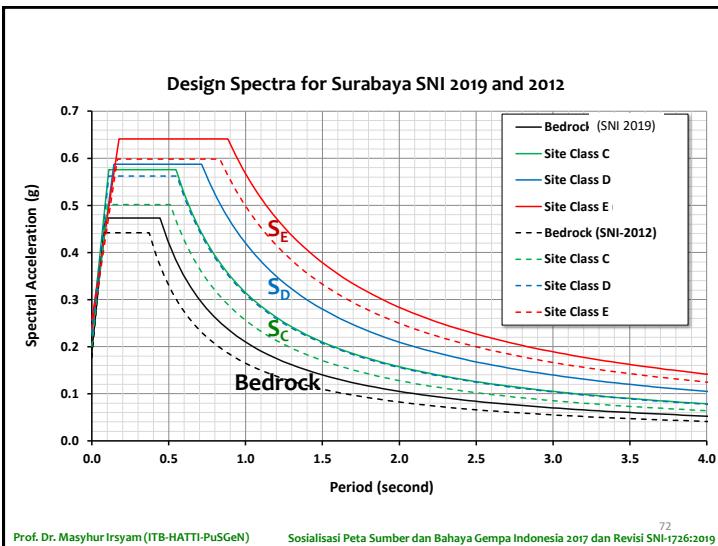
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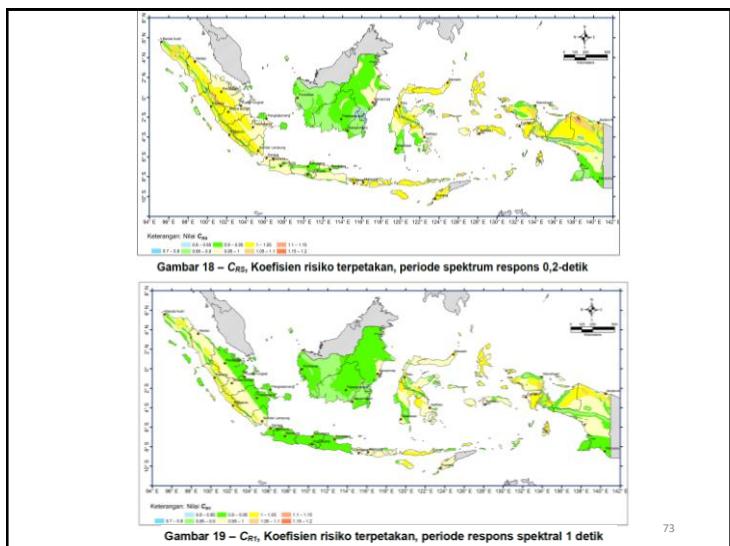
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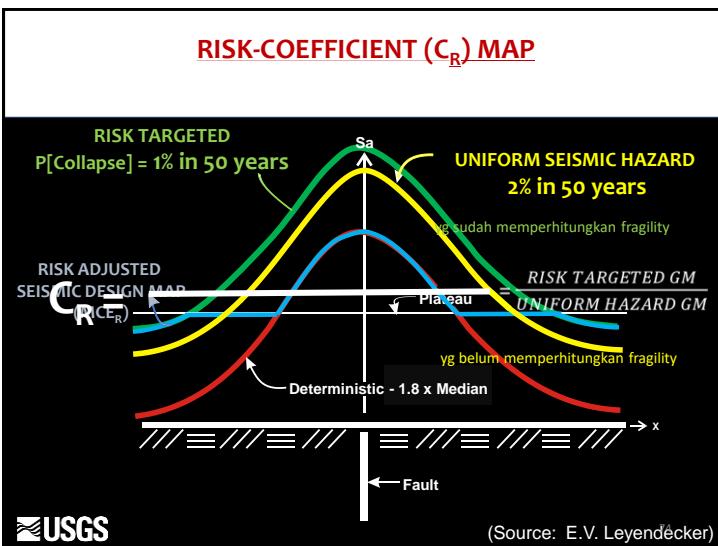
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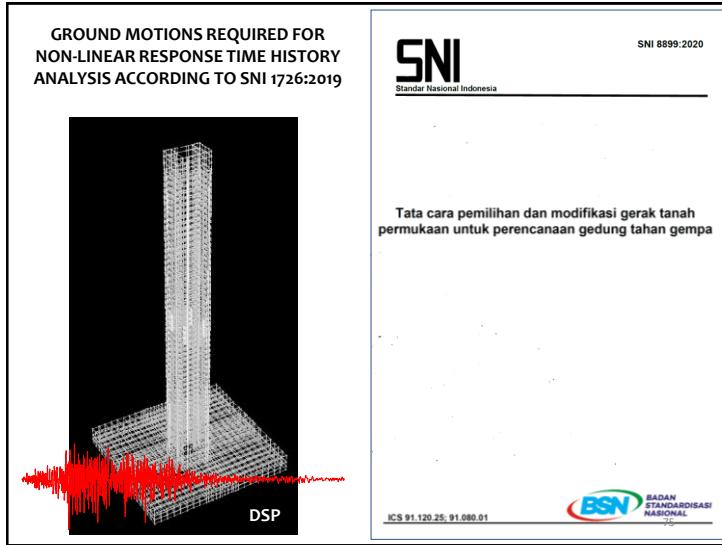
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